INFRARISK, 4TH SUMMER SCHOOL WORKSHOP, 18TH JULY 2018

The hydraulic modeling of extreme flood events

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Moselle River, France



IRSTEA

National Research Institute of Science and Technology For Environment and Agriculture

- 9 regional centers (1550 people)
- Lyon-Villeurbanne center (240 people)
- Research topics in Lyon
 - Hazards and risks linked to the water cycle
 - Quality of aquatic systems and ecological restoration
 - Biological and ecological responses to contamination of the aquatic environment



OUTLINE OF THE PRESENTATION

1. Context: the FlowRes project (2015-2018)

2. Experimental modelling of extreme flows

- Flows through emergent tree models
- Flows through emergent & weakly submerged house models
- Mixing layers for overbank flows
- 3. Numerical modelling of extreme flows
 - 1D & 3D modellings of flows through tree models
 - 1D+ & 3D modellings of overbank flows

4. Perspectives (2019-2022)



1. CONTEXT: FLOWRES PROJECT (2015-2018)





'Predicting the flow in the floodplains with evolving land occupations during extreme flood events'



AGENCE NATIONALE DE LA RECHERCHE



Partners: Irstea Lyon, LMFA, IMFT, EDF (France); UCL, Lab. Châtelet (Belgium); IST, LNEC (Portugal); UiA (Norway); K.I.T (Germany); Aberdeen Univ. (UK); ENPA (Algeria), CNR (France), AKKA Technologies (France), NIT Rourkela (India) Coordinator: S. Proust

1. CONTEXT: FLOWRES PROJECT (2015-2018)

'Predicting the flow in the floodplains with evolving land occupations during extreme flood events'

- European Flood Directive:
 - 1. Extreme events: return period $T \ge 1000$ -year
 - 2. Flood hazard maps in areas with significant flood risk (with **flow depths** and **velocities**)
- No field data for such events to validate the models
- Physical processes partly driven by land occupation

FLOW RESistance affected by





- Lateral and longitudinal changes in land occupation, i.e. in hydraulic roughness
- 2. Variable submergence of the roughness elements (function of flow depth, and of the type of element)
- **3. Inhomogeneity** of the roughness elements (increases with T)

Objective

Improving the assessment of **flow depth** and **velocity** on the floodplains by

1) analysing the physical processes for various land occupations and discharge magnitudes, focusing on the flow resistance associated with meadows, trees and houses

2) assessing if the existing modelling practices used for T ~ 100-year are still valid to predict flows with T \geq 1000-year

Methodology

1) Laboratory experiments



2) Comparison experimental data / numerical modelling (1D to 3D).



3) Field case: floods of the Doubs river at Besançon

2. EXPERIMENTAL MODELLING OF EXTREME FLOWS

irstea Cmfa

Experiments conducted at:

Irstea flume (18 m x 3 m)



LNEC flume (10 m x 2 m)



LMFA flume (8 m x 1.2 m)



IMFT flume (24 m x 1.1 m)

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LABORATÓRIO NACIONAL DE ENGENHARIA CIVIL



Irstea flume (18 m x 1 m)



2.1. FLOWS THROUGH EMERGENT TREE MODELS

Compound channel



> video



Meadow over the floodplains Tree models + meadow

Trees + meadow \rightarrow meadow (& vice versa)

• Single channel







2.1. FLOWS THROUGH EMERGENT TREE MODELS

Key issues: effects on flow structure of

- the floodplain (FP) land use (and its variation)
- the discharge magnitude
- the interaction 'flow in the main channel (MC) / flow in the FPs'

1st example of result: effect of the FP land use on the MC flow



2.1. FLOWS THROUGH EMERGENT TREE MODELS

<u>2nd example</u>: effect of discharge magnitude on water surface oscillations (induced by vortex shedding)



> video

Q = 13 L/s, Flow depth D = 99 mm



Q = 7 L/s, Flow depth D = 56 mm



From emergent to weakly submerged house models





Key issues:

- Effect of submergence D / h on flow structure and water surface oscillations
- Comparison streamwise uniform / non-uniform flows

<u>Result:</u> lock-in process (see video)

Chetibi et al. (2018); in review

LMFA flume



Key issues:

Effect of the frontal density of roughness elements λ_f on

- The drag force exerted on an element
- The bed friction

for various discharge magnitudes

Hydrodynamic balance







LMFA flume







Ludena et al. (2017), *IAHR* Ludena et al. (2018), *in prep.*

Example of results:

- Origin of flow
 resistance:
 friction or drag ?
- 1D formulation of the discharge



- Bed friction influent only for small densities of obstacles ($\lambda_f < 0.02$)



1D formulation of the discharge

Key issues: effects on flow structure of • submergence D / h

• modified submergence $\beta = (D-h) / \ell$

for h / l = 1; 3; 6

IMFT flume



h



2.3. MIXING LAYERS FOR OVERBANK FLOWS



Lab. exp. of Sellin (1964) $\rightarrow U_{max}$ reduced by -26% in the main channel

Key issue: conditions of existence of 2D coherent structures (2D CSs)?



 $\lambda = 0.7$

 $\lambda = 0.3$

- Existence of 2D CSs if dimensionless shear $\lambda = (U_2-U_1)/(U_2+U_1) \ge 0.3$
- \bullet 2D CSs magnitude increases with λ

Proust et al. (2017), *WRR* Proust & Nikora (2018), *in prep.*

2.3 - MIXING LAYERS FOR OVERBANK FLOWS

LNEC



Key issue: assessing the drag force on square cylinder, under the effect of a compoundchannel shear layer

$$C_d = \frac{2R_x}{\rho U_0^2 d}$$

where R_x is the drag force per unit submerged length at level *z* and U_0 is the mean velocity upstream the cylinder

Test	d	h _{fp}	h _{mc}	h _r	Re _d	
	(m)	(m)	(m)	(m)	(-)	
S0	0.045	0.045	0.145	0.31	8365	
S1	0.045	0.045	0.145	0.31	12032	

 h_{fp} : flow depth in the floodplain

 h_{mc} : flow depth in the main channel

$$h_r = h_{fp} / h_{mc}$$



Gymnopoulos et al. (2018), in review

2.3 - MIXING LAYERS FOR OVERBANK FLOWS

Result:

Studies	Flow type	Re _d	C _d	
		(-)	(-)	
S 0	Compound channel	8365	2.06	
S1	Compound channel	12032	2.00	
Yen and Yang, 2011	Air flow	6300	1.86	
Norberg, 1993	Air flow	13000	2.15	
Yen and Liu, 2011	Air flow	21000	2.06	
Lyn et al., 1995	Water tunnel	21400	2.10	
Robertson, 2016	Open channel	10000-22000	2.11	

marginally reduced C_d of the cylinder in shear flow (S1), comparatively to the case with uniformly distributed approach velocity (S0)

3.1 - 1D NUMERICAL MODELLING OF FLOWS THROUGH TREE MODELS (d) (e)





Bed friction Volume drag force (a = 0.81, C_D = 1.2) $\frac{\partial H}{\partial x} \left(1 - \frac{Q^2}{gB^2H^3} \right) = S_0 - \frac{n^2 Q^2}{H^{10/3}B^2} - \frac{aC_D Q^2}{2gH^2B^2}$ (9)

Analytical solution if bed friction is negligibleDupuis et
al. 2016,
 H_{up} $\frac{H - H_{dw}}{H_{up}} - \operatorname{atanh} \frac{H}{H_{up}} + \operatorname{atanh} \frac{H_{dw}}{H_{up}} = S_0 \frac{x}{H_{up}}$ (11)Dupuis et
al. 2016,
Env. Fluid.
Mech.

3.1 - 3D NUMERICAL MODELLING OF FLOWS THROUGH TREE MODELS

- CFD code Open FOAM
- Turbulence modelling:
 - *k*-ω SST-SAS model

(hybrid RANS/LES model)





3.1 - 3D MODELLING OF FLOWS THROUGH TREE MODELS

Mean flow: base vortices in a transverse plane



Are being compared with experimental data



Chatelain & Proust (2018), in *prep.*

3.2 - 1D+ MODELLING OF OVERBANK FLOWS



Independent Subsections Method (ISM)

Proust et al. (2009), WRR

$$\left(1 - \frac{U_r^2}{gh_r}\right) \frac{dh_r}{dx} = S_o - S_r^f - S_r^D + \frac{\tau_{rm}h_r}{\rho gA_r} + \frac{q_{rm}(2U_r - U_{int,rm})}{gA_r}$$

Bed friction Turbulent mixing (1)
Volume drag force
$$\left(1 - \frac{U_l^2}{gh_l}\right) \frac{dh_l}{dx} = S_o - S_l^f - S_l^D + \frac{\tau_{lm}h_l}{\rho gA_l} + \frac{q_{lm}(2U_l - U_{int,lm})}{gA_l}$$
(2)
$$\left(1 - \frac{U_m^2}{gh_m}\right) \frac{dh_m}{dx} = S_o - S_m^f - \frac{\tau_{lm}h_l}{\rho gA_m} - \frac{\tau_{rm}h_r}{\rho gA_m} \dots$$
(3)
$$\dots - \frac{q_{lm}(2U_m - U_{int,lm})}{gA_m} - \frac{q_{rm}(2U_m - U_{int,rm})}{gA_m}$$

$$\frac{dQ_m}{dx} + \frac{dQ_l}{dx} + \frac{dQ_r}{dx} = 0$$

(4)

Mass

e

3.2 - 1D+ MODELLING OF OVERBANK FLOWS



3.2 - 3D MODELLING OF OVERBANK FLOWS



- Open FOAM
- *k*-ω SST-SAS model



Development of the 2D Kelvin-Helmholtz type coherent structures (2D CSs)



Figure 18: Qf-8 - Instantaneous iso-value of λ_2 criterion in the horizontal plane located at z = 132.5 mm.

3.2 - 3D MODELLING OF OVERBANK FLOWS

Focus on the 2D CSs length scales

Two-point velocity measurement \rightarrow Space time correlation function



$$R_{ij}^{k}\left(k,\epsilon_{k},\tau\right) = \frac{\overline{u_{i}'\left(k,t\right)u_{j}'\left(k+\epsilon_{k},t+\tau\right)}}{\sqrt{\overline{u_{i}'^{2}\left(k\right)}} \overline{u_{j}'^{2}\left(k+\epsilon_{k}\right)}}$$

Experimental longitudinal scale at given x-position



Simulated longitudinal scale vs. x-position



4.1 - PERSPECTIVES (2019-2022): MIXING LAYERS IN NON-COMPOUND CHANNELS

What drives the existence of Kelvin-Helmholtz instabilities in <u>free-surface shear flows</u>?

FOCUSING ON FLOWS IN

• Compound channel (2013-2018)



S. Proust, V. Dupuis, C. Berni, V. Nikora, J. Fernandes, J. Leal, N. Rivière, Y. Peltier, A. Paquier

• Single channel (2018-2019)



• Composite channel (2018-2019)



S. Proust, V. Nikora, C. Berni

4.2 - PERSPECTIVES (2019-2022): URBAN FLOOD PROCESSES

- Project DEUFI (Detailing Urban Flood Impact) funded by the ANR (2019-2022)
 - Influence of lateral interfaces in urban flood processes (PhD of M. Meija from nov. 2018- nov. 2021)



Block scaleWP1 Laboratory experimentsWP1bImpact of opening porosity on flowdepth and velocity in the block andneighboring streets.Effect of the urbanistic organizationinside the block (courtyards vs. smallbuildings) on flow pattern and timeof submersion TRiverLy, MURI model (5.4<u>m×3.8m).</u>Mapping of flow depth andvelocity for steady flows and timescale for unsteady flows

4.3 - PERSPECTIVES (2019-2022): UNSTEADY FLOW EFFECTS ON OVERBANK FLOWS

PhD of Y. Kaddi (2018-2021): '1D+ Modelling (ISM) of a complex hydraulic network. Application to the operational forecasting of high flow events and floods related to the sizing of civil engineering structures.'

• July-Nov 2018: validation of ISM under unsteady flow conditions





4.4 - PERSPECTIVES (2019-2022): 1D+ ISM FOR COMPLEX NETWORKS

 Then, application to the Rhône river (PhD half-funded by the Compagnie Nationale du Rhône)



Confluence Rhône / Ardêche, 2003

100-year return period discharge

 Make ISM operational for networks with junctions



Thank you for your attention !

.... any questions ?