

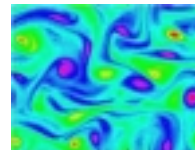
Elastoplastic Analysis in Structural Engineering

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Wavelet Methods in Elastoplasticity



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Outline

1. Classical Theory of Plasticity
2. Elastoplasticity in Structural Engineering
 - 1D problems
 - 2D problems
 - 3D problems
3. *Standard* Numerical Techniques
4. Hybrid-Mixed Formulations
5. Approximation of plastic parameters
6. Perturbation Methods
7. Examples of Application



Classical Theory of Plasticity

Equilibrium conditions

$$\mathbf{D} \boldsymbol{\sigma} + \mathbf{b} = \mathbf{0} \quad \text{in } V$$

$$\mathbf{N} \boldsymbol{\sigma} = \mathbf{t}_\gamma \quad \text{on } \Gamma_\sigma$$

Compatibility conditions

$$\boldsymbol{\varepsilon} = \boldsymbol{\varepsilon}_e + \boldsymbol{\varepsilon}_p = \mathbf{D}^* \mathbf{u} \quad \text{in } V$$

$$\mathbf{u} = \mathbf{u}_\gamma \quad \text{on } \Gamma_u$$

Elasticity

$$\boldsymbol{\varepsilon}_e = \mathbf{f} \boldsymbol{\sigma} + \boldsymbol{\varepsilon}_\theta$$



Classical Theory of Plasticity

Plasticity

$$\phi_* = \phi_*(\boldsymbol{\sigma}, \bar{\varepsilon}_p) - \bar{\sigma}_e$$

$$\phi_* \leq 0$$

$$\phi_* = \sqrt{\boldsymbol{\sigma}^t \mathbf{M} \boldsymbol{\sigma}} - \bar{\sigma}_e$$

$$d\phi_* = \left(\frac{\partial \phi}{\partial \sigma_{ij}} \right)^t d\sigma_{ij} + \frac{\partial \phi}{\partial \bar{\varepsilon}_p} d\bar{\varepsilon}_p$$

$$d\phi_* = \mathbf{n}_*^t d\boldsymbol{\sigma} + \frac{\partial \phi}{\partial \bar{\varepsilon}_p} d\bar{\varepsilon}_p$$

$$d\phi_* = \mathbf{n}_*^t d\boldsymbol{\sigma} - h_* d\varepsilon_*$$

Classical Theory of Plasticity

Plasticity

$$d\varepsilon_{ij}^{(p)} = \frac{\partial \phi}{\partial \sigma_{ij}} d\varepsilon_*$$

$$d\varepsilon_* \geq 0$$

$$d\varepsilon_p = \mathbf{n}_* d\varepsilon_*$$

$$\mathbf{n}_* = \frac{\mathbf{M}\boldsymbol{\sigma}}{\bar{\sigma}_e}$$

$$\phi_* d\varepsilon_* = 0 \quad , \quad d\phi_* d\varepsilon_* = 0$$



Classical Theory of Plasticity

Integration of constitutive relations

$$\Delta \boldsymbol{\varepsilon}_p = \mathbf{n}_* \Delta \varepsilon_* + \mathbf{R}_\varepsilon^*$$

$$\Delta \phi_* = \mathbf{n}_*^t \Delta \boldsymbol{\sigma} - h_* \Delta \varepsilon_* + R_\phi^*$$

$$\phi_* + \Delta \phi_* \leq 0$$

$$\Delta \varepsilon_* \geq 0$$

$$\phi_* \Delta \varepsilon_* = 0$$

$$\Delta \phi_* \Delta \varepsilon_* = 0$$



Elastoplasticity in Structural Engineering

Some problems

- Definition of the geometry
- Definition of loading conditions
- Characterisation of material behaviour (concrete, soils, ...)



Elastoplasticity in Structural Engineering

1D problems



Elastoplasticity in Structural Engineering

2D problems



Elastoplasticity in Structural Engineering

3D problems



Standard Numerical Techniques

Main features

- Use of displacement finite element formulations
- Control of plasticity on sets of selected points
- Use of Newton-Raphson algorithms to solve the non-linear governing system
- Use of a wide range of return-mapping algorithms

- Linearisation of the yield surface
- Use of Mathematical Programming Techniques
- Discretisation of plastic parameter field



Standard Numerical Techniques

Some limitations

- Insufficient quality of the stress field estimates
- Computation of *unsafe* estimates for the collapse load
- Position of control points
- Mesh dependency

Hybrid-Mixed Formulations

Approximation criteria

$$\boldsymbol{\sigma} = \mathbf{S}_v \mathbf{X} + \boldsymbol{\sigma}_p \text{ in } V$$

$$\mathbf{u} = \mathbf{U}_v \mathbf{q}_v + \mathbf{u}_p \text{ in } V$$

$$\mathbf{u} = \mathbf{U}_\gamma \mathbf{q}_\gamma \text{ on } \Gamma_\sigma$$

$$\Delta \boldsymbol{\varepsilon}_* = \mathbf{P}_* \Delta \mathbf{e}_* \text{ in } V$$



Hybrid-Mixed Formulations

Energy conditions

$$\mathbf{e}^t \mathbf{X} = \int \boldsymbol{\varepsilon}^t (\boldsymbol{\sigma} - \boldsymbol{\sigma}_p) dV$$

$$\mathbf{q}_v^t \mathbf{Q}_v = \int \mathbf{b}^t (\mathbf{u} - \mathbf{u}_p) dV$$

$$\mathbf{q}_\gamma^t \mathbf{Q}_\gamma = \int \mathbf{u}^t \mathbf{t}_\gamma d\Gamma_\sigma$$

$$\Delta \mathbf{e}_*^t \Delta \Phi_* = \int \Delta \boldsymbol{\varepsilon}_*^t \Delta \phi_* dV$$

Hybrid-Mixed Formulations

Energy conditions

$$e = \int \mathbf{S}_v^t \boldsymbol{\varepsilon} dV$$

$$Q_v = \int \mathbf{U}_v^t \mathbf{b} dV$$

$$\Delta \Phi_* = \int \mathbf{P}_*^t \Delta \phi_* dV$$

$$Q_\gamma = \int \mathbf{U}_\gamma^t \mathbf{t}_\gamma d\Gamma_\sigma$$

Hybrid-Mixed Formulations

Equilibrium conditions

$$\mathbf{Q}_\gamma = \int \mathbf{U}_\gamma^t \mathbf{t}_\gamma d\Gamma\sigma = \int \mathbf{U}_\gamma^t \bar{\mathbf{t}}_\gamma \lambda d\Gamma\sigma = \mathbf{a}_e \lambda$$

$$\begin{bmatrix} -\mathbf{A}_v^t & 0 \\ \mathbf{A}_\gamma^t & -\mathbf{a}_e \end{bmatrix} \begin{bmatrix} \Delta \mathbf{X} \\ \Delta \lambda \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Compatibility conditions

$$\begin{bmatrix} -\mathbf{A}_v & \mathbf{A}_\gamma \\ 0 & -\mathbf{a}_e^t \end{bmatrix} \begin{bmatrix} \Delta \mathbf{q}_v \\ \Delta \mathbf{q}_\gamma \end{bmatrix} = \begin{bmatrix} \Delta \mathbf{e}_e + \Delta \mathbf{e}_p \\ -\Delta \bar{w} \end{bmatrix}$$

$$\Delta \bar{w} = \mathbf{a}_e^t \Delta \mathbf{q}_\gamma = \int \bar{\mathbf{t}}_\gamma^t \mathbf{U}_\gamma \Delta \mathbf{q}_\gamma d\Gamma\sigma = \int \bar{\mathbf{t}}_\gamma^t \Delta \mathbf{u} d\Gamma\sigma$$



Hybrid-Mixed Formulations

Elasticity

$$\mathbf{e}_e = \mathbf{F}\mathbf{X} + \mathbf{e}_\theta$$

Plasticity

$$\begin{bmatrix} \Delta \mathbf{e}_p \\ \Delta \Phi_* \end{bmatrix} = \begin{bmatrix} \mathbf{0} & \mathbf{N}_* \\ \mathbf{N}_*^t & -\mathbf{H}_* \end{bmatrix} \begin{bmatrix} \Delta \mathbf{X} \\ \Delta \mathbf{e}_* \end{bmatrix} + \begin{bmatrix} \mathbf{R}_e^* \\ \mathbf{R}_\Phi^* \end{bmatrix}$$

$$\Phi_* + \Delta \Phi_* \leq 0$$

$$\begin{cases} \Phi_*^t \Delta \mathbf{e}_* = 0 \\ \Delta \Phi_*^t \Delta \mathbf{e}_* = 0 \end{cases}$$

$$\Delta \mathbf{e}_* \geq 0$$



Hybrid-Mixed Formulations

Governing system - elastoplastic analysis

$$\begin{bmatrix} \mathbf{F} & \mathbf{A}_v & -\mathbf{A}_\gamma & \mathbf{N}_* & 0 \\ \mathbf{A}_v^t & 0 & 0 & 0 & 0 \\ -\mathbf{A}_\gamma^t & 0 & 0 & 0 & \mathbf{a}_e \\ \mathbf{N}_*^t & 0 & & -\mathbf{H}_* & 0 \\ 0 & 0 & \mathbf{a}_e^t & 0 & 0 \end{bmatrix} \begin{bmatrix} \Delta \mathbf{X} \\ \Delta \mathbf{q}_v \\ \Delta \mathbf{q}_\gamma \\ \Delta \mathbf{e}_* \\ \Delta \lambda \end{bmatrix} = \begin{bmatrix} -\mathbf{R}_e^* \\ 0 \\ 0 \\ \Delta \Phi_* - \mathbf{R}_\Phi^* \\ \Delta \bar{w} \end{bmatrix}$$

$$\Phi_* + \Delta \Phi_* \leq 0; \quad \begin{cases} \Phi_*^t \Delta \mathbf{e}_* = 0 \\ \Delta \Phi_*^t \Delta \mathbf{e}_* = 0. \end{cases} \quad ; \quad \Delta \mathbf{e}_* \geq 0$$

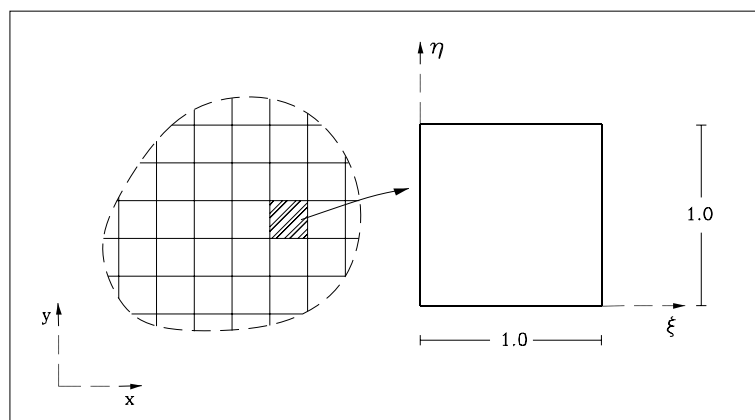
Hybrid-Mixed Formulations

Governing system - elastic analysis

$$\begin{bmatrix} \mathbf{F} & \mathbf{A}_v & -\mathbf{A}_\gamma \\ \mathbf{A}_v^t & 0 & 0 \\ -\mathbf{A}_\gamma^t & 0 & 0 \end{bmatrix} \begin{bmatrix} \mathbf{X} \\ \mathbf{q}_v \\ \mathbf{q}_\gamma \end{bmatrix} = \begin{bmatrix} \mathbf{e}_\gamma - \mathbf{e}_\theta \\ -\mathbf{Q}_v \\ -\mathbf{Q}_\gamma \end{bmatrix}$$

Approximation of plastic parameters

$$\Delta \varepsilon_* = \mathbf{P}_* \Delta \mathbf{e}_* \text{ in } V$$



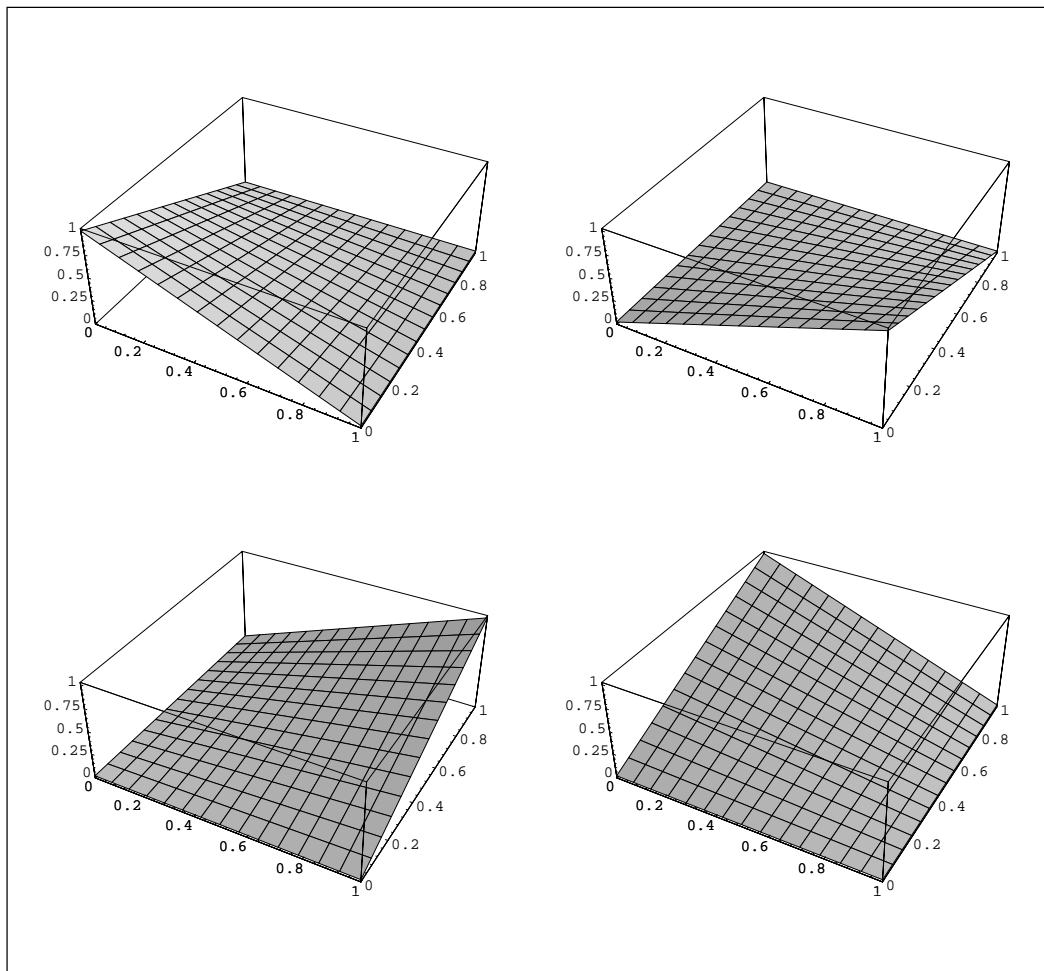
Approximation of plastic parameters

$$f_*^{(1)} = (1 - \xi)(1 - \eta)$$

$$f_*^{(2)} = \xi(1 - \eta)$$

$$f_*^{(3)} = \xi\eta$$

$$f_*^{(4)} = (1 - \xi)\eta$$



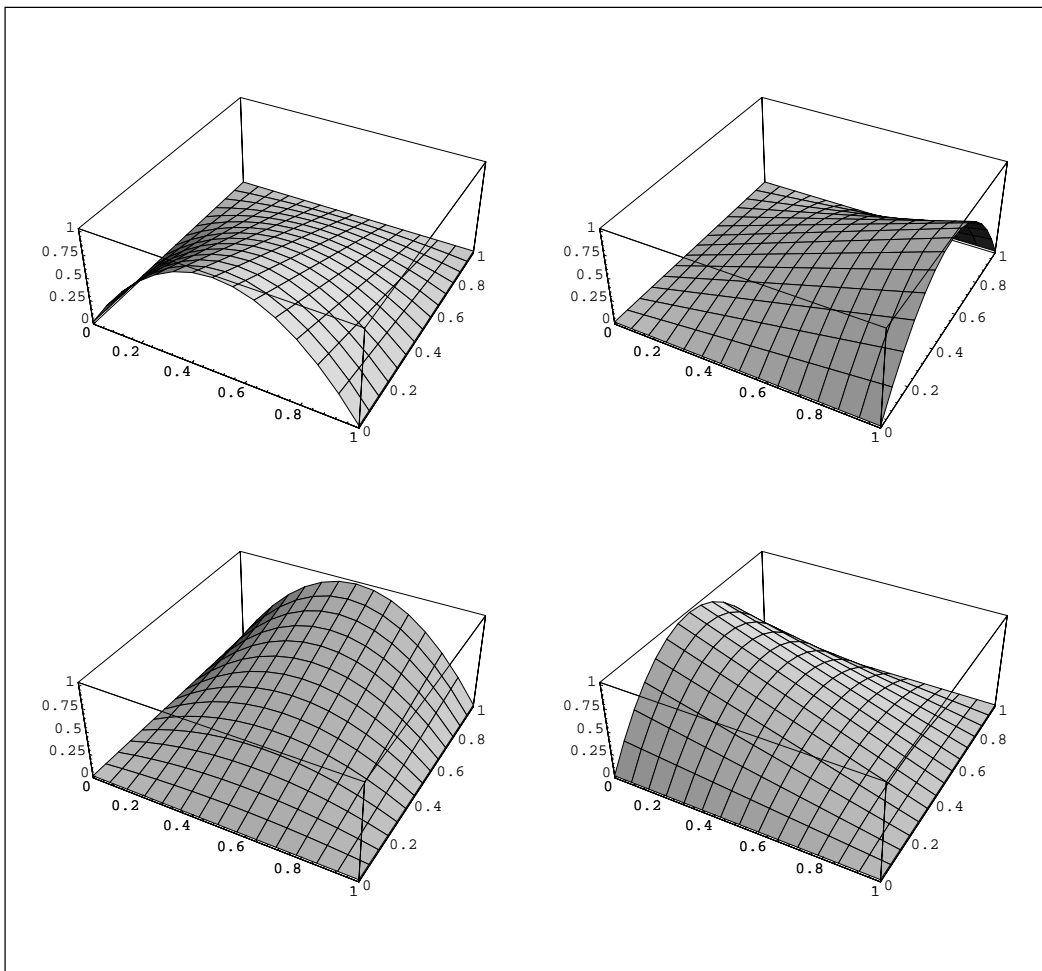
Approximation of plastic parameters

$$f_*^{(5)} = 4\xi(\xi - 1)(\eta - 1)$$

$$f_*^{(6)} = 4\xi\eta(1 - \eta)$$

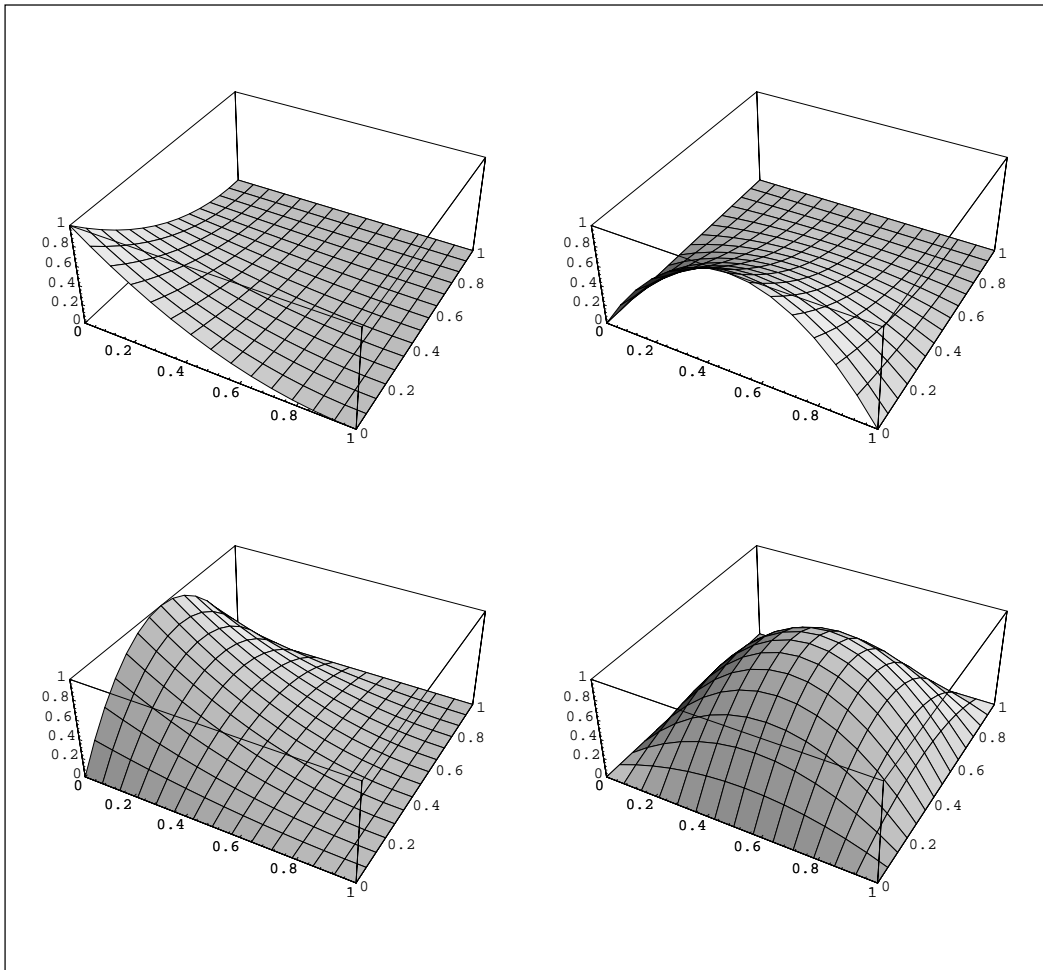
$$f_*^{(7)} = 4\xi\eta(1 - \xi)$$

$$f_*^{(8)} = 4\eta(\xi - 1)(\eta - 1)$$



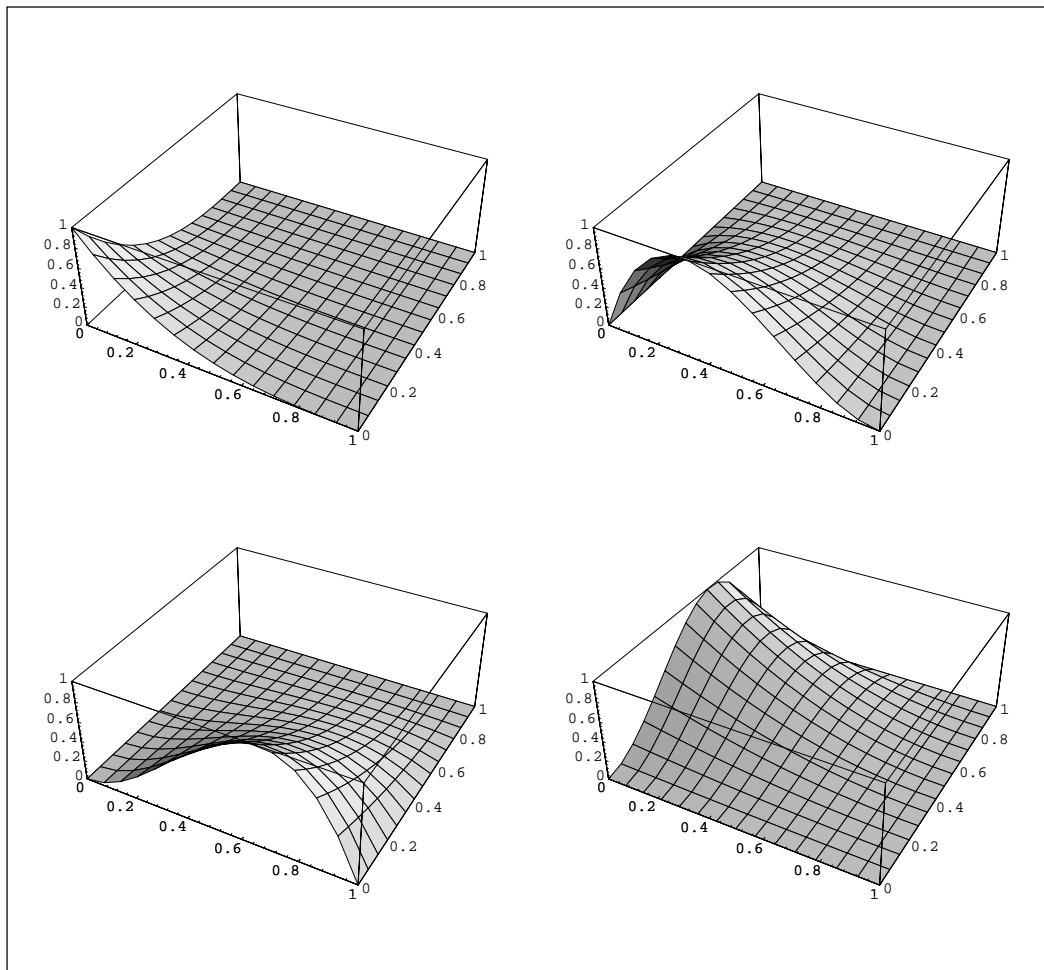
Approximation of plastic parameters

Quadratic approximation



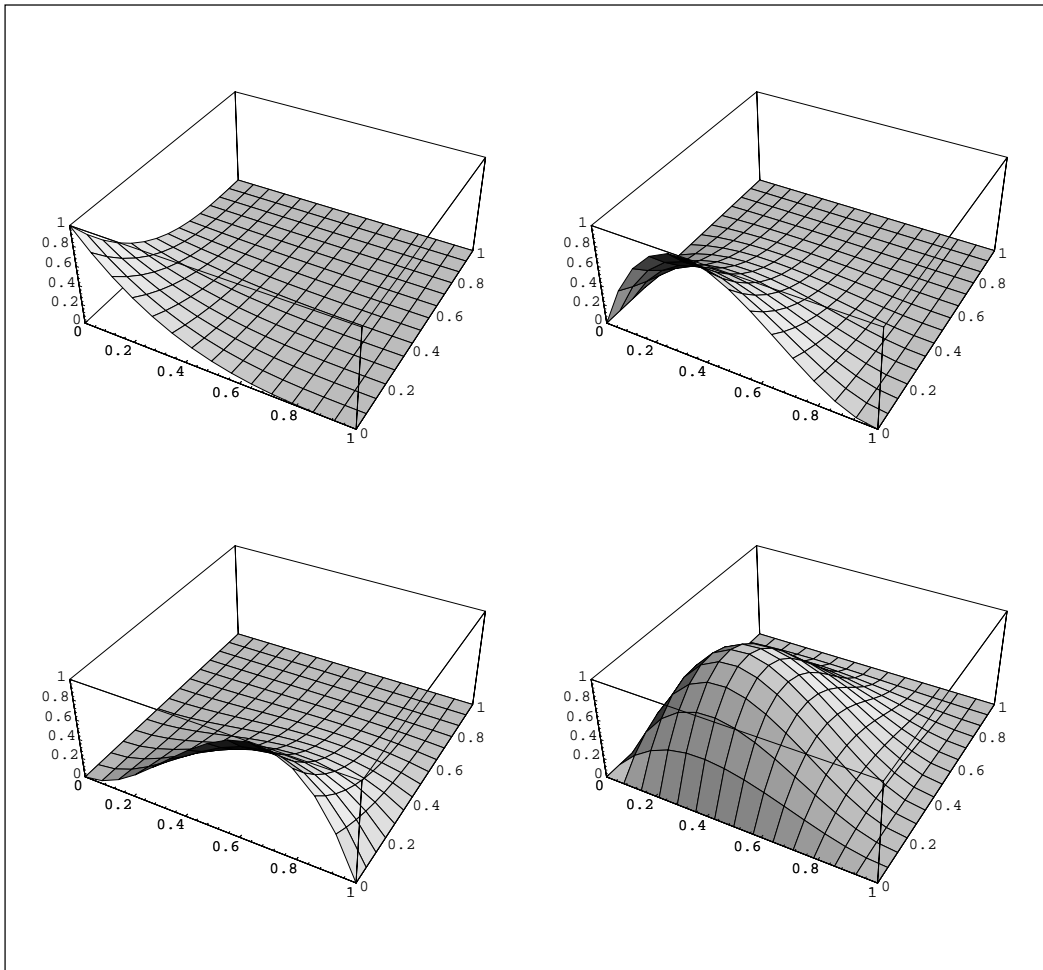
Approximation of plastic parameters

Cubic approximation



Approximation of plastic parameters

Quartic approximation



Perturbation Methods

Basic idea

$$\Delta v = \sum_{n=1}^{\infty} \frac{v^{(n)} \tau^n}{n!}$$

$$\begin{bmatrix} \mathbf{F} & \mathbf{A}_v & -\mathbf{A}_\gamma & \mathbf{N}_* & 0 \\ \mathbf{A}_v^t & 0 & 0 & 0 & 0 \\ -\mathbf{A}_\gamma^t & 0 & 0 & 0 & \mathbf{a}_e \\ \mathbf{N}_*^t & 0 & 0 & -\mathbf{H}_* & 0 \\ 0 & 0 & \mathbf{a}_e^t & 0 & 0 \end{bmatrix} \begin{bmatrix} \mathbf{X}^{(n)} \\ \mathbf{q}_v^{(n)} \\ \mathbf{q}_\gamma^{(n)} \\ \mathbf{e}^{(n)} \\ \lambda^{(n)} \end{bmatrix} = \begin{bmatrix} -\mathbf{R}_e^{*(n)} \\ 0 \\ 0 \\ -\mathbf{R}_\Phi^{*(n)} \\ \bar{w}^{(n)} \end{bmatrix}$$

Perturbation Methods

Definition of residual vectors

$$\phi_*^{(n)} = \mathbf{n}_*^t \boldsymbol{\sigma}^{(n)} + R_\phi^{*(n)}$$

$$R_\phi^{*(1)} = 0$$

$$R_\phi^{*(2)} = \boldsymbol{\sigma}^{(1)t} \mathbf{M} \boldsymbol{\sigma}^{(1)} - (\phi^{(1)})^2$$

$$R_\phi^{*(3)} = 3\boldsymbol{\sigma}^{(1)t} \mathbf{M} \boldsymbol{\sigma}^{(2)} - 3\phi^{(1)}\phi^{(2)}$$

$$R_\phi^{*(4)} = 4\boldsymbol{\sigma}^{(1)t} \mathbf{M} \boldsymbol{\sigma}^{(3)} \\ + 3\boldsymbol{\sigma}^{(2)t} \mathbf{M} \boldsymbol{\sigma}^{(2)} - 3(\phi^{(2)})^2 - 4\phi^{(1)}\phi^{(3)}$$

Perturbation Methods

Definition of residual vectors

$$\boldsymbol{\varepsilon}_p^{(n)} = \mathbf{n}_* \boldsymbol{\varepsilon}_*^{(n)} + \mathbf{R}_\varepsilon^{*(n)}$$

$$\mathbf{R}_\varepsilon^{*(1)} = 0$$

$$\mathbf{R}_\varepsilon^{*(2)} = \mathbf{n}_*^{(1)} \boldsymbol{\varepsilon}_*^{(1)}$$

$$\mathbf{R}_\varepsilon^{*(3)} = 2\mathbf{n}_*^{(1)} \boldsymbol{\varepsilon}_*^{(2)} + \mathbf{n}_*^{(2)} \boldsymbol{\varepsilon}_*^{(1)}$$

$$\begin{aligned} \mathbf{R}_\varepsilon^{*(4)} &= 3\mathbf{n}_*^{(1)} \boldsymbol{\varepsilon}_*^{(3)} \\ &+ 3\mathbf{n}_*^{(2)} \boldsymbol{\varepsilon}_*^{(2)} + \mathbf{n}_*^{(3)} \boldsymbol{\varepsilon}_*^{(1)} \end{aligned}$$

Perturbation Methods

Step length

1.

$$\Phi_*^{(i)} + \Phi_*^{(1,i)}\tau + \Phi_*^{(2,i)}\frac{\tau^2}{2} + \Phi_*^{(3,i)}\frac{\tau^3}{6} = 0$$

2.

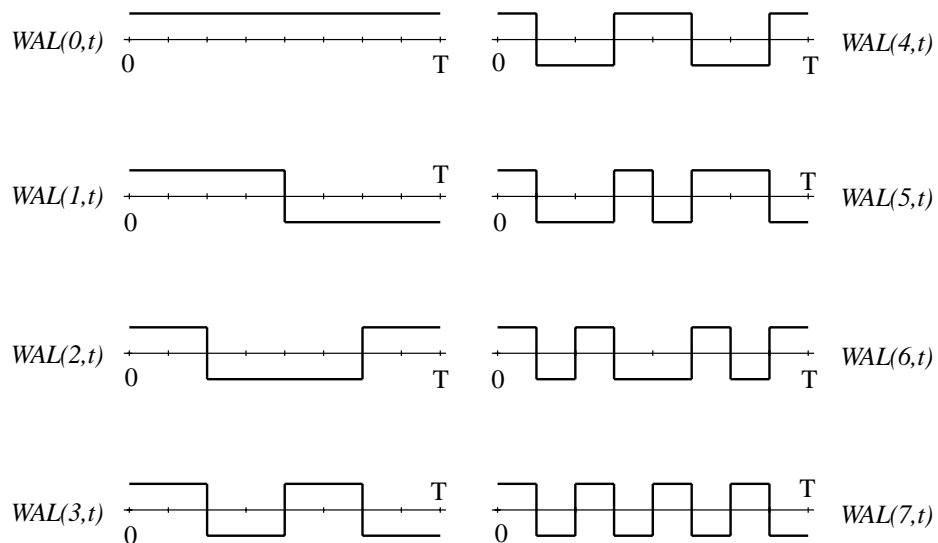
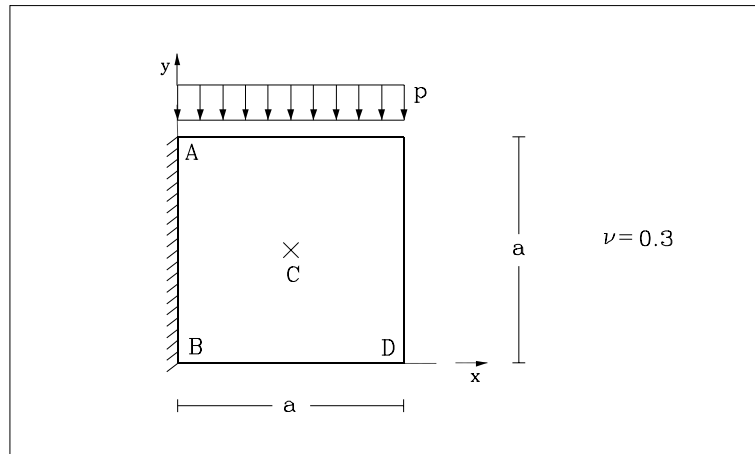
$$\tau_j \leq \left(\frac{TOL \times n!}{|v_j^{(n)}|} \right)^{\frac{1}{n}}$$

3.

$$\frac{\partial \Delta e_*}{\partial \tau} \geq 0 \Rightarrow e_*^{(1)} + e_*^{(2)}\tau + e_*^{(3)}\frac{\tau^2}{2} \geq 0$$

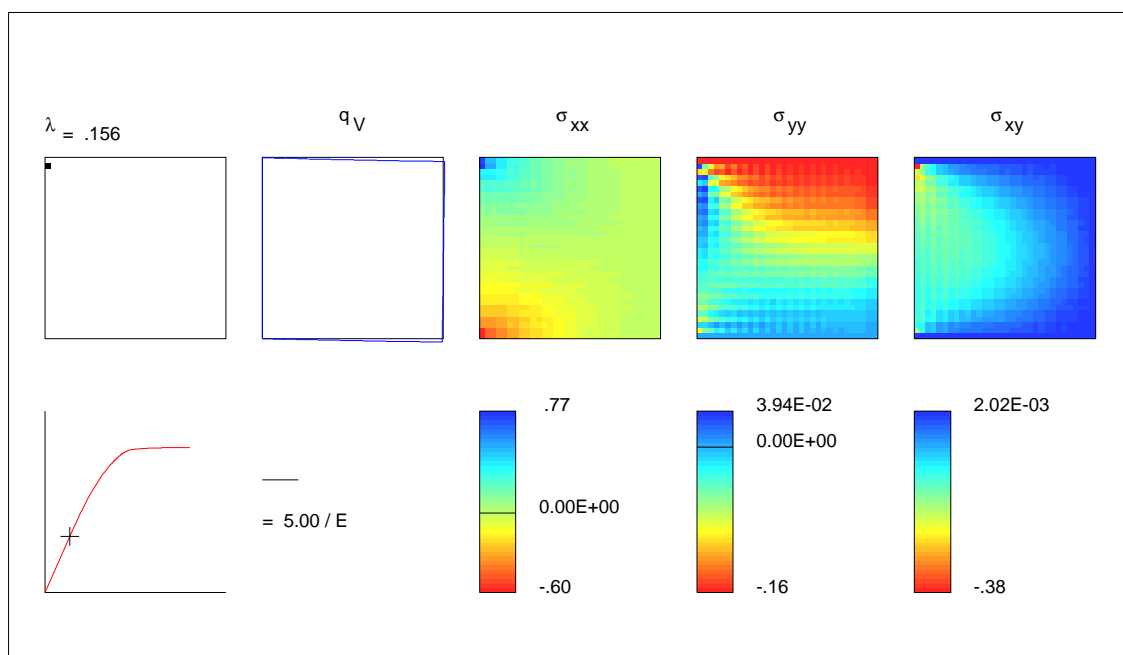


Examples of Application

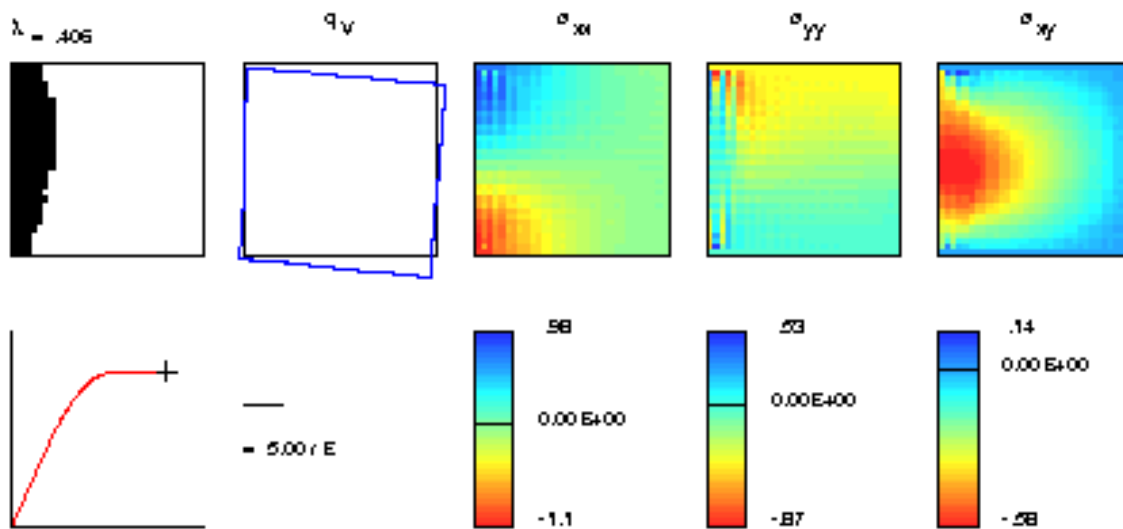
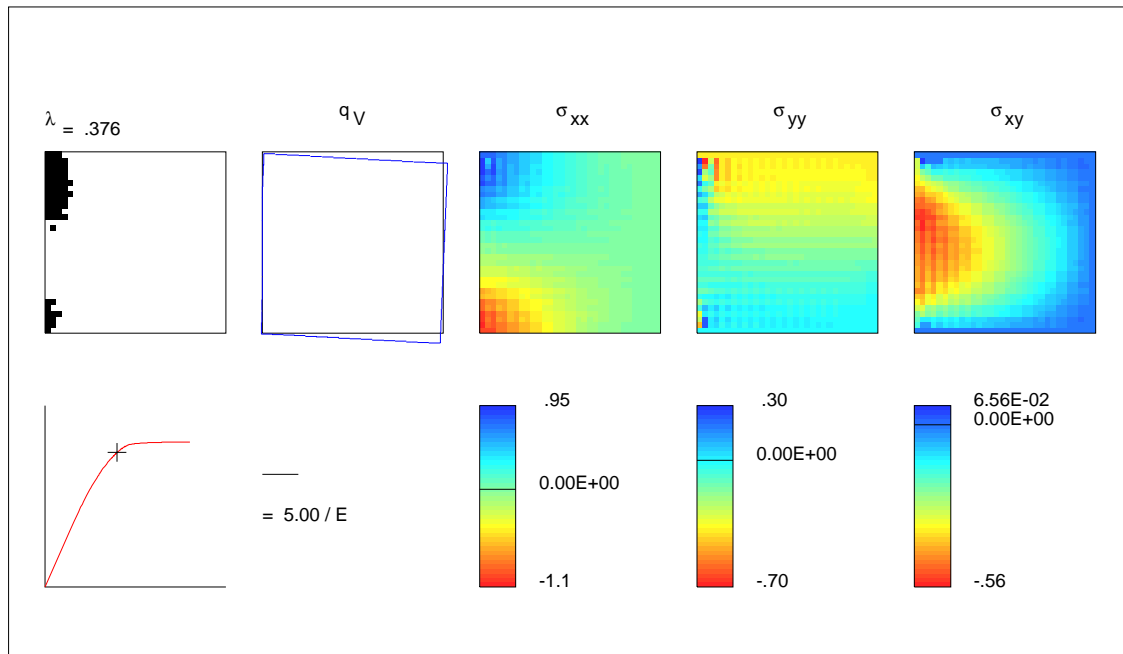


Examples of application

32×32 critical cell mesh

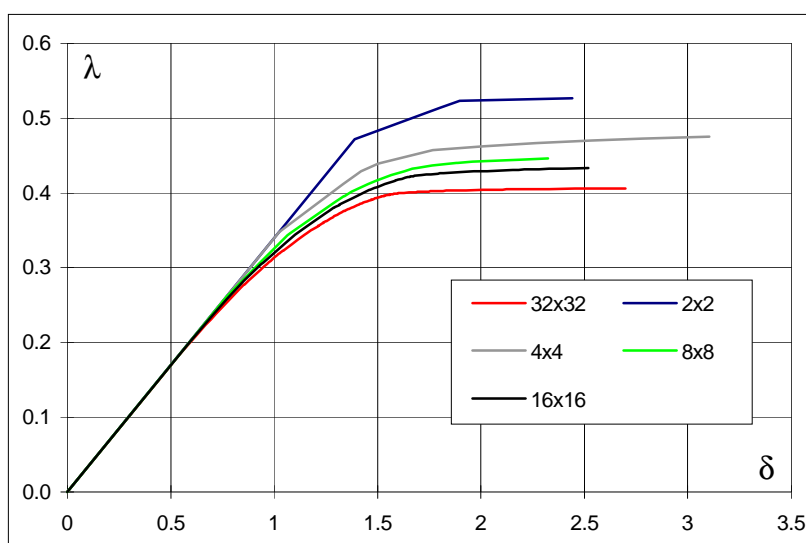


Examples of application

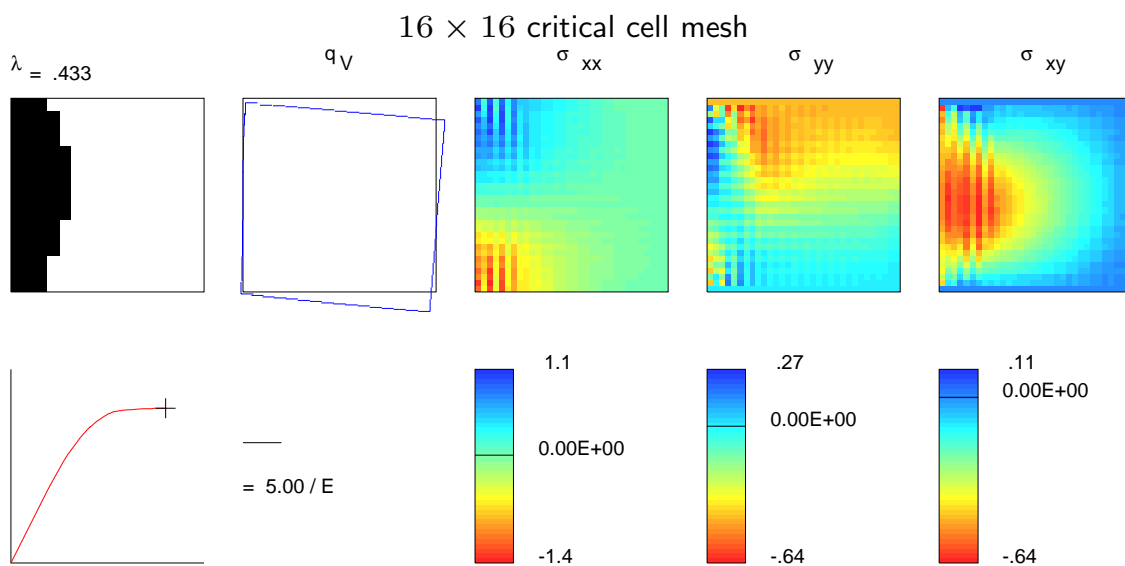
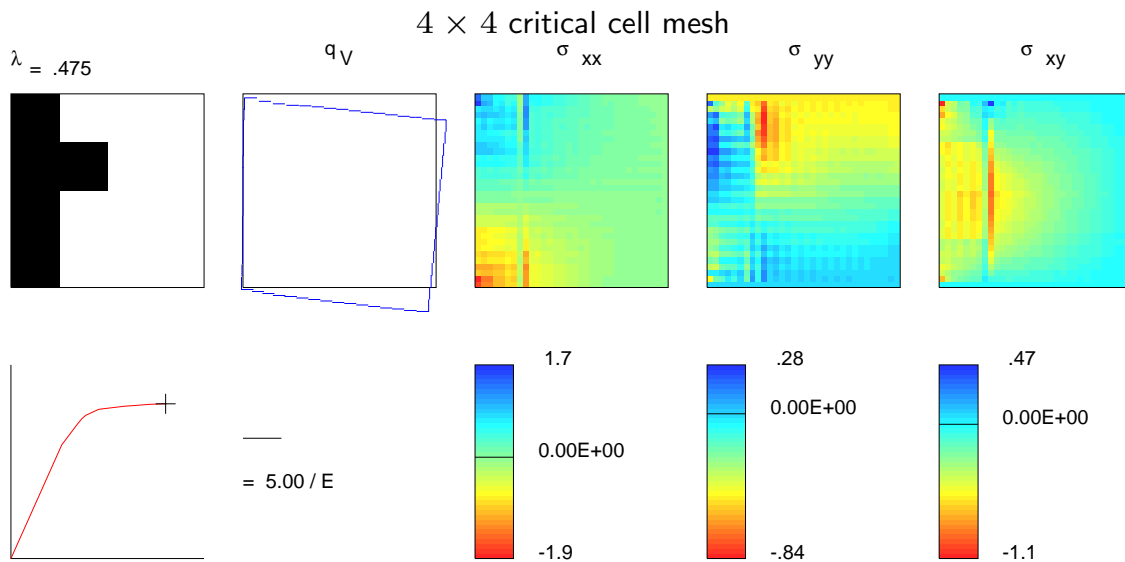


Examples of application

critical cells	loading steps	λ_c
2×2	4	0.527
4×4	10	0.475
8×8	20	0.446
16×16	68	0.433
32×32	180	0.406

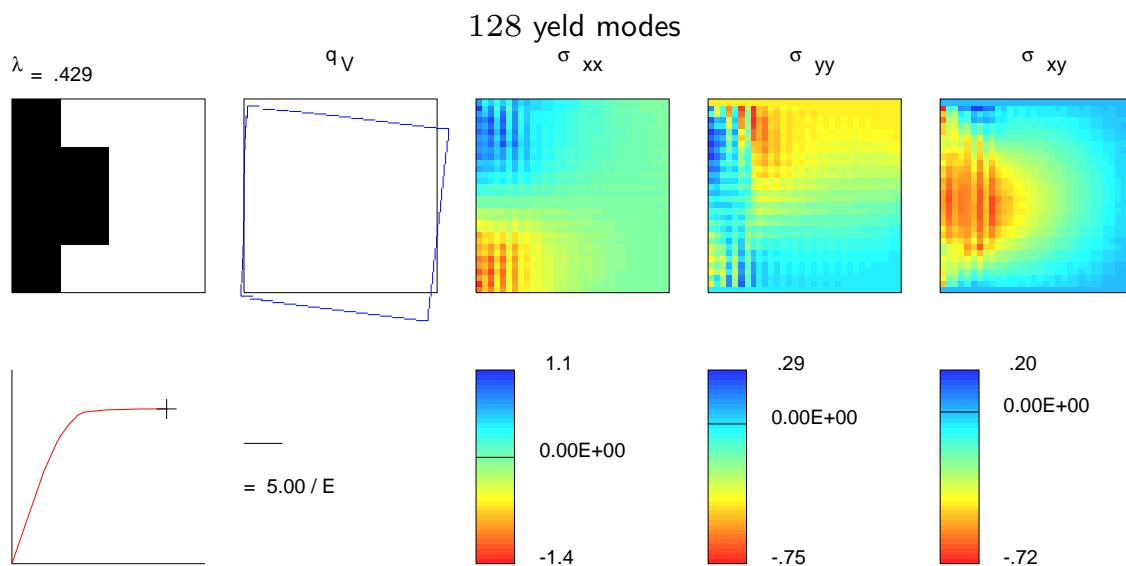


Examples of application

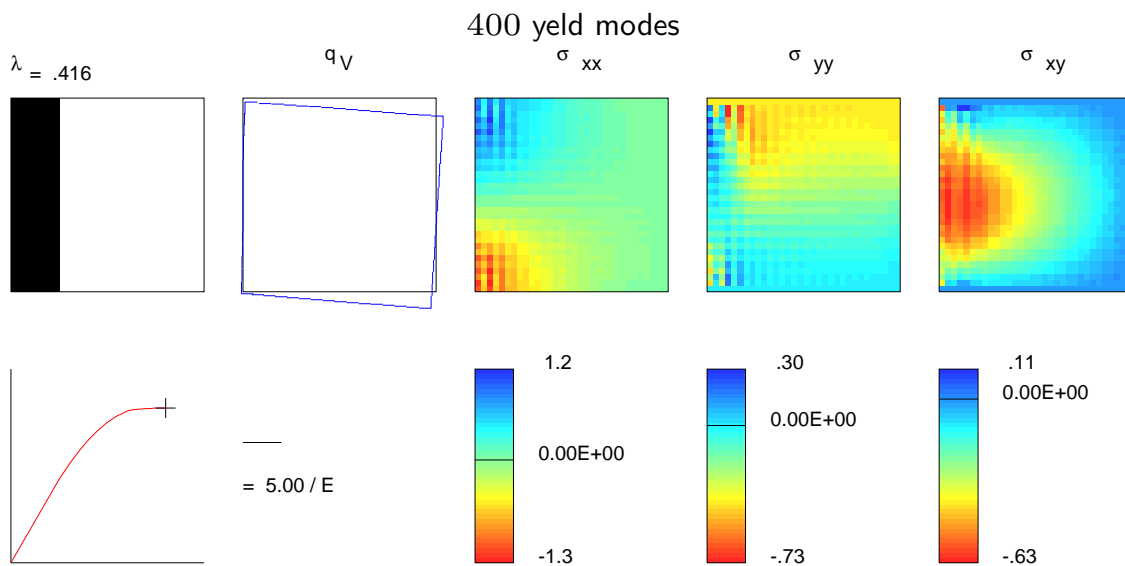


Examples of application

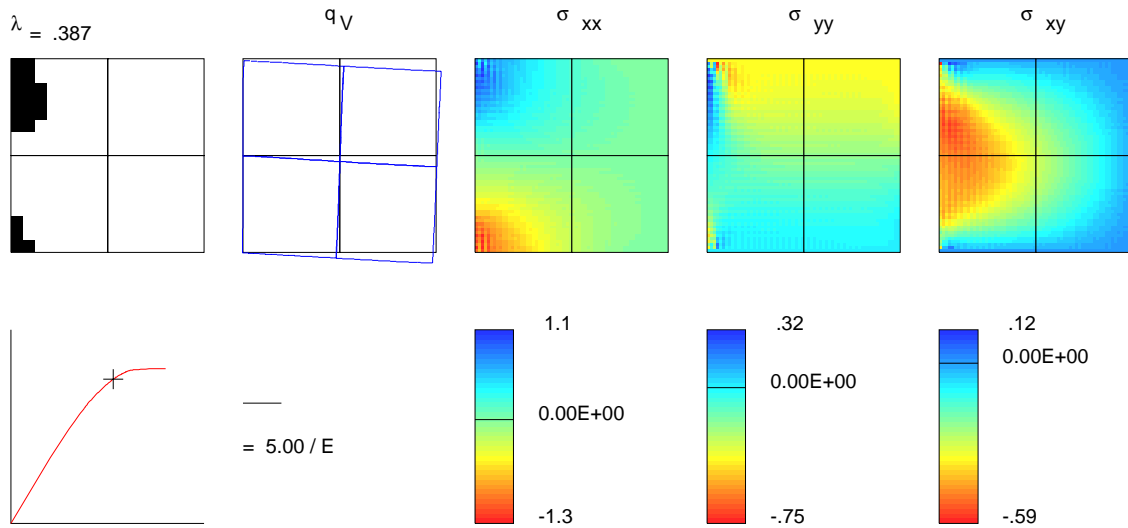
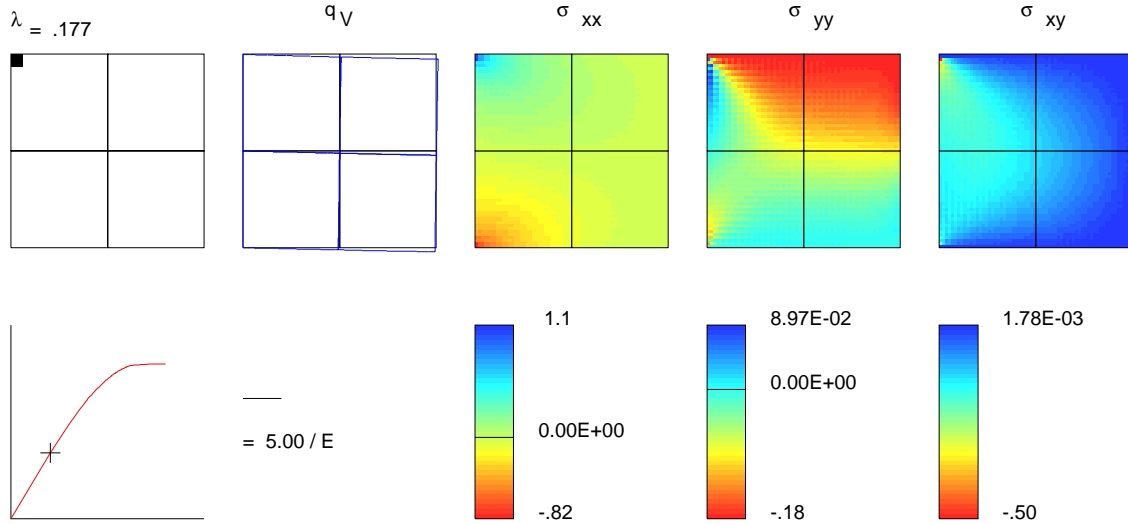
<i>Degree</i>	<i>yield modes</i>	<i>loading steps</i>	λ_c
<i>constant</i>	16	10	0.476
<i>linear</i>	64	15	0.437
<i>parabolic (8)</i>	128	20	0.429
<i>parabolic (9)</i>	144	27	0.428
<i>cubic (12)</i>	192	41	0.423
<i>quartic</i>	400	39	0.416



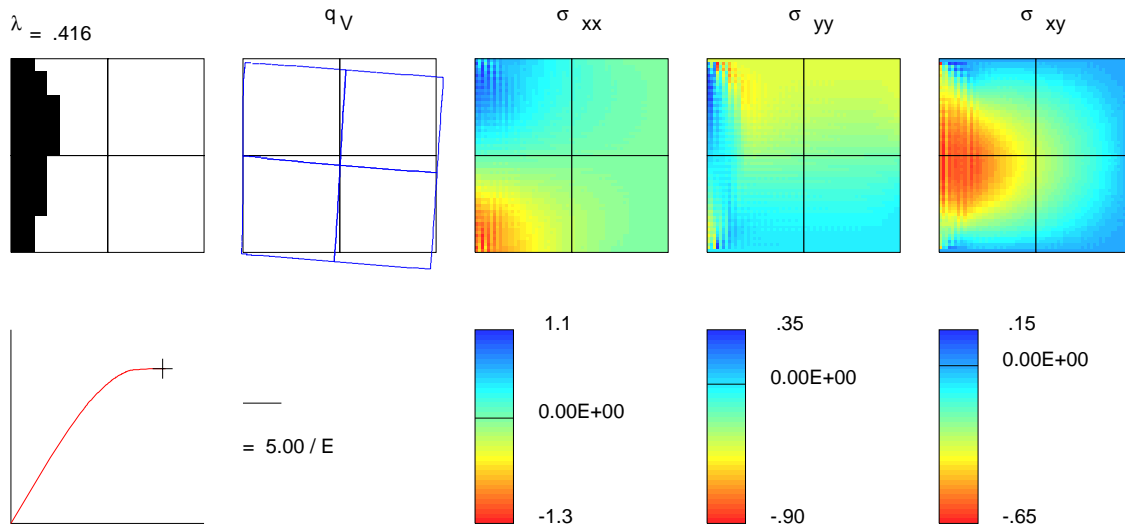
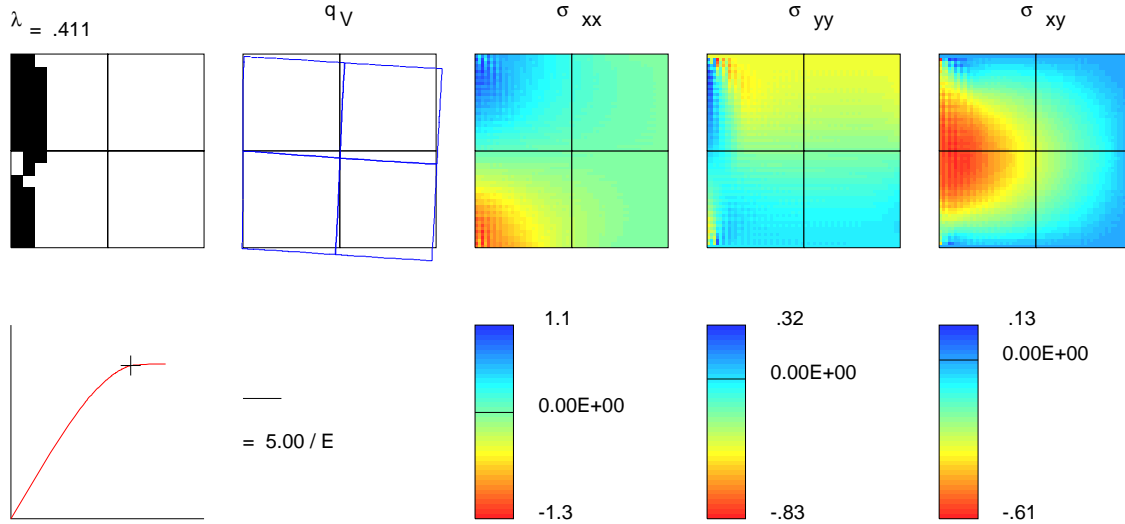
Examples of application



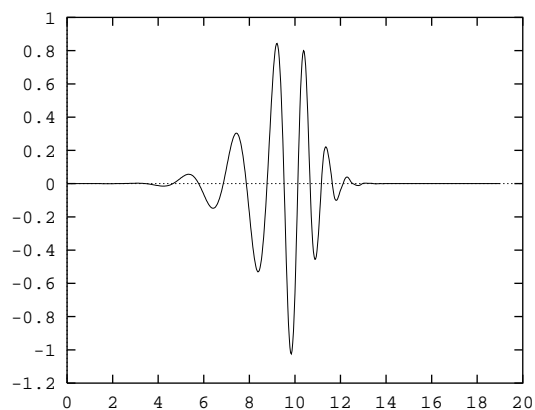
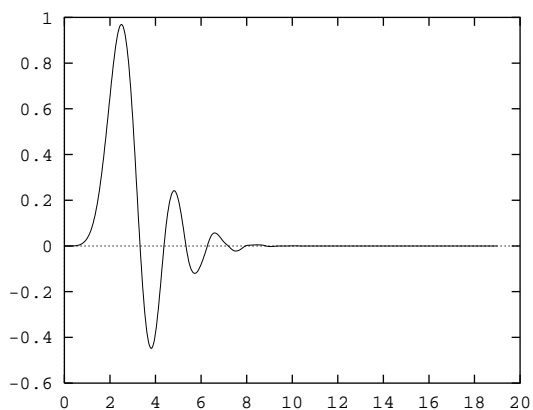
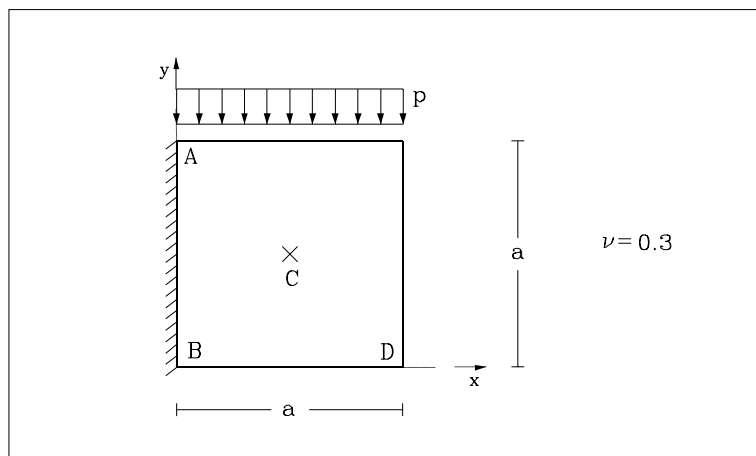
Examples of application



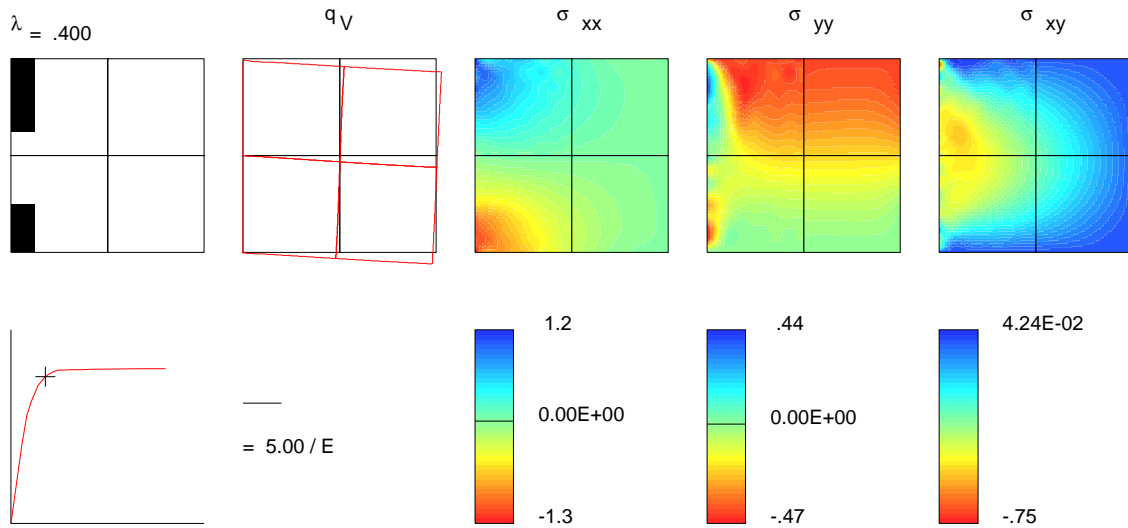
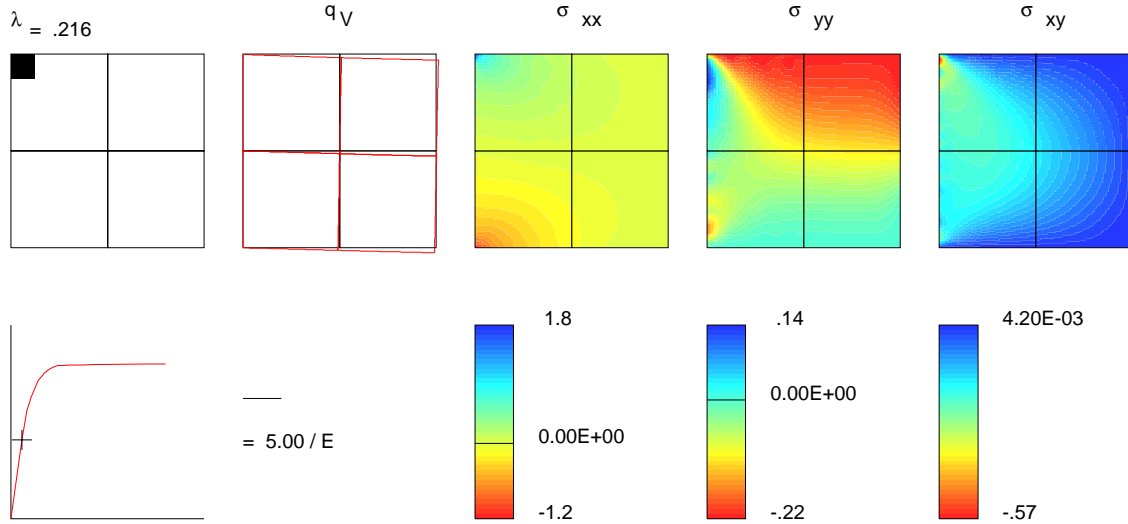
Examples of application



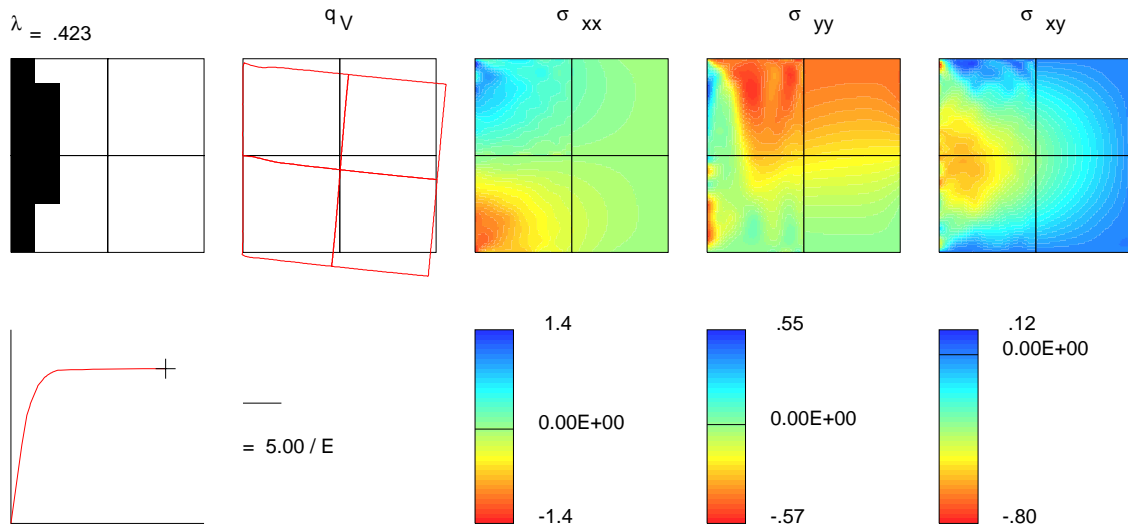
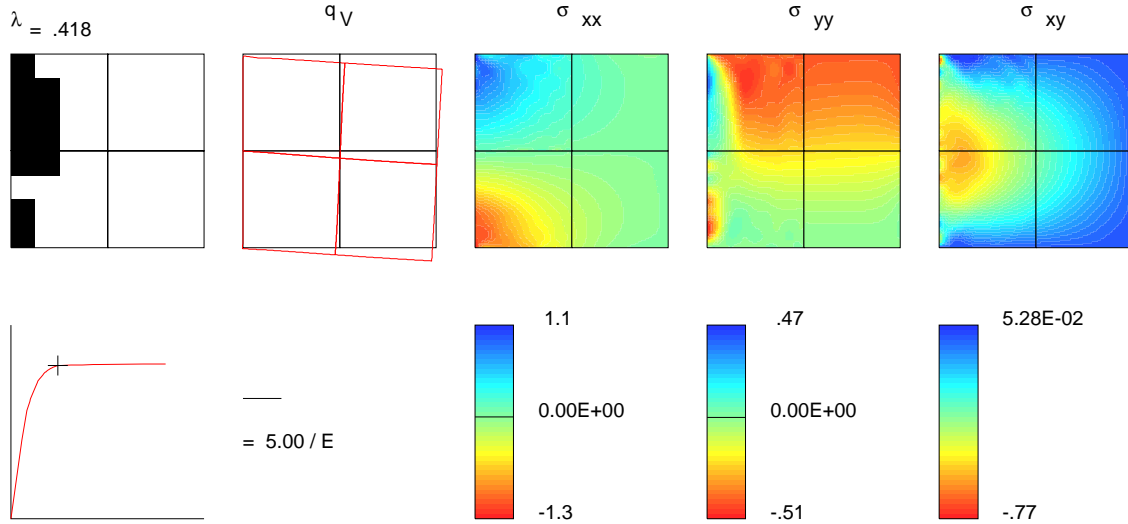
Examples of application



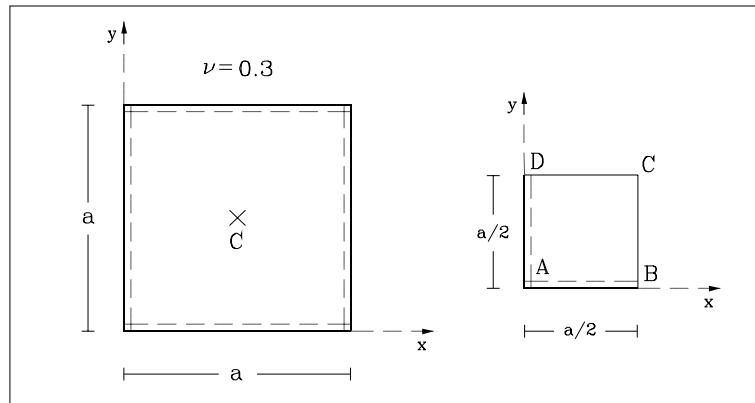
Examples of application



Examples of application



Examples of application



Examples of application

	<i>Degree</i>				
c. cells	0	1	2	3	4
1 × 1	0.529	0.481	0.461	0.448	0.439
2 × 2	0.492	0.437	0.431	0.418	0.418
4 × 4	0.441	0.422	0.415	0.408	0.408

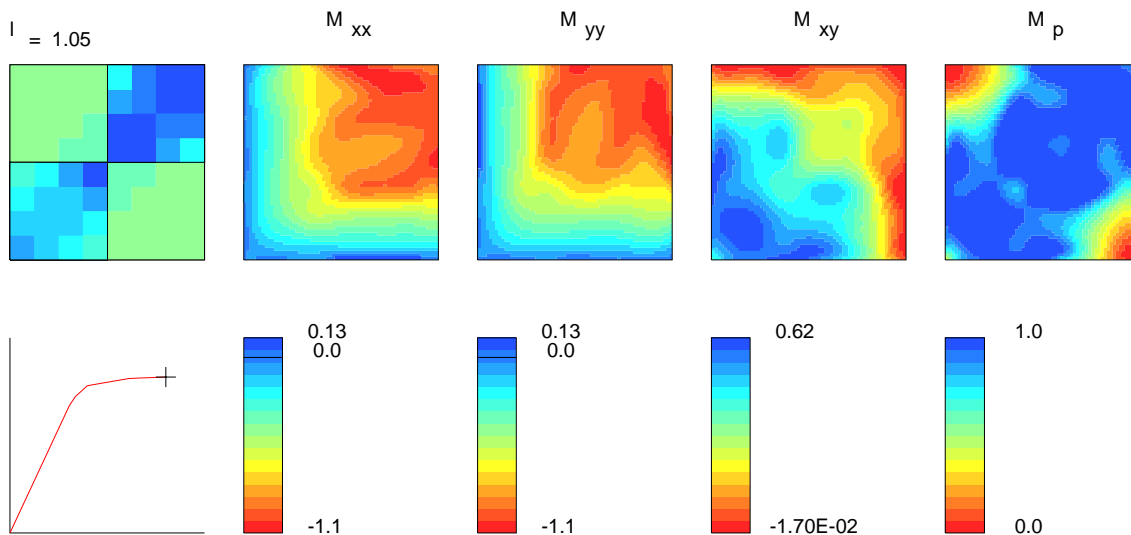
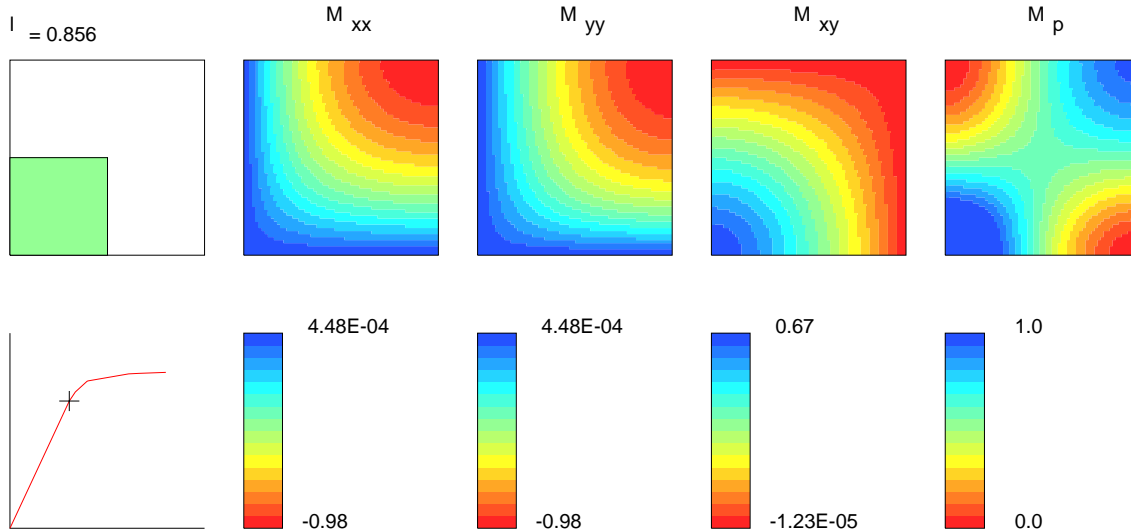
$$\lambda^* = \frac{\lambda a^2}{6 m_0}$$

Model	<i>Degree</i>	n_{modes}	λ_e^*	λ_c^*
Present	0	1	0.9503	1.1208
	1	4	0.8560	1.0522
	2	8	0.8150	1.0495
	3	12	0.7929	1.0465
	4	25	0.7796	1.0436
Ang and Lopez			0.74	1.031
Belytschko and Velebit			0.810	1.068

$$1.036 < \lambda_c^* < 1.106$$



Examples of application



Examples of application

<i>Plastic cells</i>	λ_e^*	λ_c^*
2×2	0.8560	1.0522
4×4	0.7778	1.0443
8×8	0.7514	1.0329

